

FILE NO.

Standard-SSV

REVISIONS

REV	DESCRIPTION	DWN	CHK	APP
8	REBRAIN TO ALTOCAD	JDA	JDM	HAA

DATE: JUL17/2006

DWG REFERENCE

ROHN

PRODUCTS

6718 WEST PLANK ROAD

PEORIA, IL 61604

TOLL FREE 800-777-ROHN

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FOUNDATION & ANCHOR TOLERANCE

DWN: CSR

CHKD: KTL

DATE: 5sep/25/1987

ENGR: XK

DRAWING NO. A810214

REV: 8

FOUNDATION AND ANCHOR TOLERANCES

ALL FOUNDATIONS

1. CONCRETE DIMENSIONS - PLUS OR MINUS 1" (25mm).

2. DEPTH OF FOUNDATION - PLUS 3" (76mm) OR MINUS 0".

3. DRILLED FOUNDATIONS OUT OF PLUMB - 1.0 DEGREE.

4. REINFORCING STEEL PLACEMENT - PER A.C.I. 301.

5. PROJECTION OF EMBEDMENTS - PLUS OR MINUS 1/8" (3mm).

6. VERTICAL EMBEDMENTS OUT OF PLUMB - 0.5 DEGREE.

7. MAXIMUM DISTANCE FROM CENTERLINE OF ANCHOR BOLTS TO CENTERLINE OF FOUNDATION - 1/24 OF PIER DIAMETER UP TO A MAXIMUM OF 2" (51mm).

8. ANCHOR BOLT SPACING - 1/16" (2mm).

9. ANCHOR BOLT CIRCLE ORIENTATION - 0.25 DEGREE.

10. ANCHOR BOLT CIRCLE DIAMETER - PLUS OR MINUS 1/16" (2mm).

SELF-SUPPORTING TOWERS

11. FACE SPREAD DIMENSION CENTER TO CENTER OF ANCHOR BOLT CIRCLES - PLUS OR MINUS 1/16" (2mm) OR 1/16" (2mm) PER 20 FT. (6m) OF FACE SPREAD.

12. MAXIMUM DIFFERENCE BETWEEN ANY TWO FOUNDATION ELEVATIONS - 1/2" (13mm).

GUYED TOWERS

13. GUY RADIUS - PLUS OR MINUS 5% OF DISTANCE SPECIFIED.

14. ANCHOR ELEVATION - PLUS OR MINUS 5% OF GUY RADIUS.

15. ANCHOR ALIGNMENT (PERPENDICULAR TO GUY RADIUS) - 1.0 DEGREE.

16. ANCHOR ROD SLOPE - PLUS OR MINUS 1.0 DEGREE.

17. ANCHOR ROD ALIGNMENT WITH GUY RADIUS PLUS OR MINUS 1.0 DEGREE.

18. ANCHOR HEAD OUT OF PLUMB - 1.0 DEGREE.

19. GUY INITIAL TENSION - PLUS OR MINUS 10% OF TENSION SPECIFIED.

NOTE: TOLERANCES IN NOTES 13 AND 14 CAN NOT OCCUR SIMULTANEOUSLY

WARNING!!

AFTER ANCHOR BOLTS ARE INSTALLED IN CONCRETE HAS TAKEN ITS INITIAL SET, ANCHOR BOLTS MUST NOT BE MOVED, BENT OR REALIGNED IN ANY MANNER. A NUT LOCKING DEVICE MUST BE INSTALLED ON ALL ANCHOR BOLTS.

10/1/2008 2:22:22 PM

10/1/2008

FILE NO.

STDPUBLIC

REVISIONS

DESCRIPTION

DWN

CHK

APP

DWG REFERENCE

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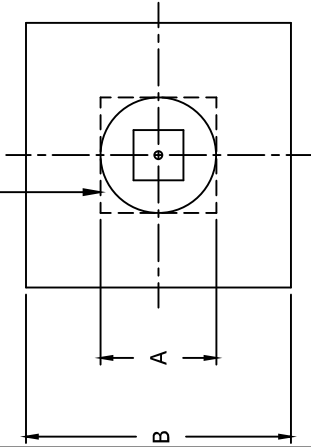
FOUNDATION
BASE PIER FOR REV. G PRESUMPTIVE CLAY

DWN: FAD
CHKD: HA
DATE: Nov/24/2009
ENGR: HA

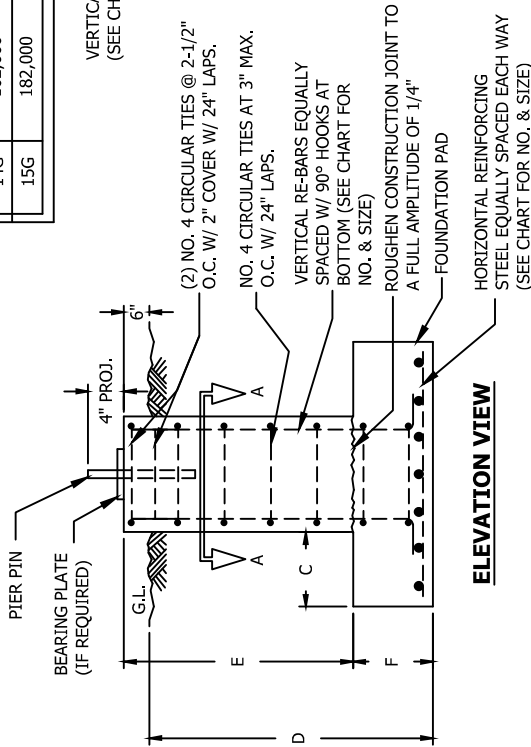
DRAWING NO. B090549
REV: 0

CONCRETE BASE SCHEDULE FOR ANSI/TIA-222-G PRESUMPTIVE CLAY SOIL											
CB NO.	TOWER BASE REACTION (LBS)	DIMENSIONS						BEARING PLATE	CONC. (CU. YDS RD PIER)	VERTICAL BARS (NO. & SIZE)	HORIZ. BARS IN PAD (NO. & SIZE)
		★ A	B	C	D	E	F				
1G	12,000	2'-6"	2'-6"	0	4'-0"	0	0	BP6	0.80	8 NO. 7	NONE
2G	17,000	3'-0"	3'-0"	0	4'-0"	0	0	BP6	1.20	10 NO. 7	NONE
3G	23,000	3'-6"	3'-6"	0	4'-0"	0	0	BP6	1.60	12 NO. 7	NONE
4G	30,000	4'-0"	4'-0"	0	4'-0"	0	0	BP6	2.10	12 NO. 8	NONE
5G	38,000	2'-0"	4'-0"	1'-0"	4'-0"	3'-3"	1'-3"	BP6	1.10	8 NO. 6	5 NO. 5
6G	48,000	2'-0"	4'-6"	1'-3"	4'-0"	3'-3"	1'-3"	BP6	1.30	8 NO. 6	6 NO. 5
7G	58,000	2'-0"	5'-0"	1'-6"	4'-6"	3'-9"	1'-3"	BP10	1.60	8 NO. 6	6 NO. 5
8G	71,000	2'-0"	5'-6"	1'-9"	4'-6"	3'-9"	1'-3"	BP10	1.80	8 NO. 6	7 NO. 5
9G	84,000	2'-0"	6'-0"	2'-0"	4'-6"	3'-6"	1'-6"	BP10	2.40	8 NO. 6	7 NO. 6
10G	99,000	2'-0"	6'-6"	2'-3"	4'-6"	3'-6"	1'-6"	BP10	2.80	8 NO. 6	8 NO. 5
11G	111,000	2'-6"	7'-0"	2'-3"	5'-0"	3'-9"	1'-9"	BP15	3.90	8 NO. 7	8 NO. 6
12G	127,000	2'-6"	7'-6"	2'-6"	5'-0"	3'-9"	1'-9"	BP15	4.30	8 NO. 7	9 NO. 6
13G	145,000	2'-6"	8'-0"	2'-9"	5'-0"	3'-9"	1'-9"	BP15	4.80	8 NO. 7	9 NO. 6
14G	162,000	3'-0"	8'-6"	2'-9"	5'-0"	3'-6"	2'-0"	BP15	6.30	12 NO. 7	9 NO. 7
15G	182,000	3'-0"	9'-0"	3'-0"	5'-0"	3'-6"	2'-0"	BP15	6.90	12 NO. 7	10 NO. 7

ALTERNATE SQUARE PIER
(SEE NOTE 3)



PLAN VIEW



ELEVATION VIEW

VERTICAL BARS EQUALLY SPACED
(SEE CHART FOR NO. & SIZE)

ALTERNATE SQUARE PIER
#4 CIRCULAR TIES 3" ON CENTERS
W/ 24" LAPS

SECTION A-A

NOTES:

- SEE TOWER ASSEMBLY DRAWING FOR FOUNDATION LAYOUT AND PART NUMBERS FOR BEARING PLATE & PIER PIN.
- SEE DRAWING NUMBER B090548 FOR STANDARD FOUNDATION NOTES.
★3. USE MIN. 2'-6" SQ. OR 3'-0" DIA. ROUND PIER WHEN BPC45G OR BPC55G IS USED.
- VERTICAL REINFORCING STEEL SHALL BE REPLACED WITH STRAIGHT BARS WHEN NO PAD IS REQUIRED.
- HORIZ. BARS IN CHART REFER ONLY TO THE BARS IN THE FOUNDATION PAD.

FILE NO.

STD/PUBLIC

REVISIONS			
REV.	DESCRIPTION	DWN.	CHK. APP.
1	REVISED	FAD	HA
2	DATE: 05/27/2009 SLOPE NOTES CHANGED	FAD	HA
3	DATE: 06/21/2010 UPDATED LAYOUT	FAD	HA

DWG REFERENCE

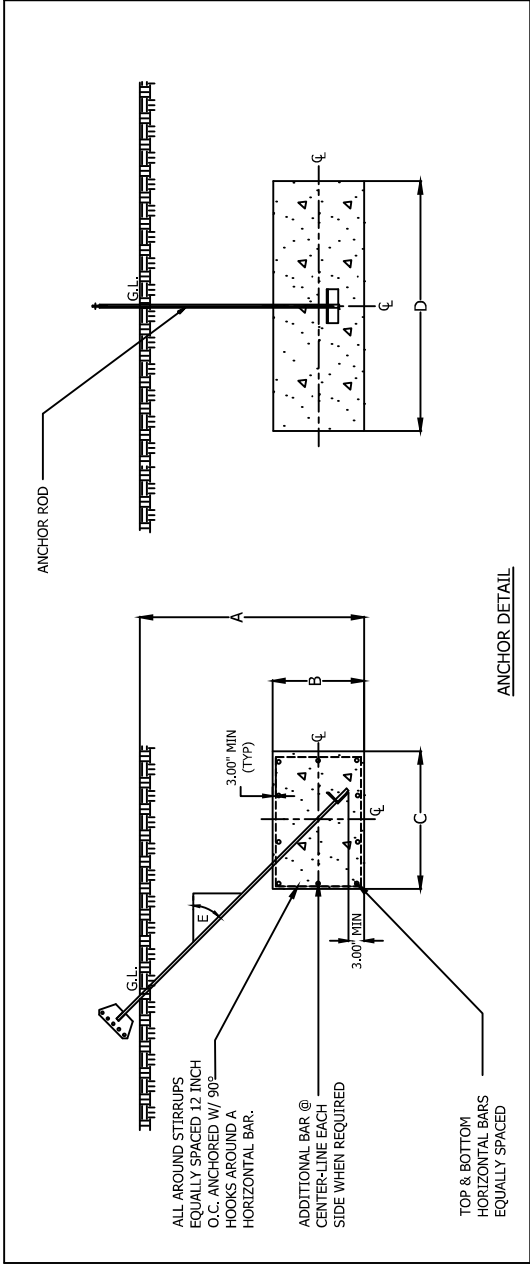
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FOUNDATION
ANCHOR BLOCK REV. G PRESUMPTIVE CLAY

DWN:	FAD	CHK'D:	HA	DATE:	Nov/24/2009
ENGR:					
DRAWING NO:	B090550				REV:
					3



GENERAL NOTES

1. SEE DRAWING NUMBER B090548 FOR STANDARD FOUNDATION NOTES.
2. ALL HORIZONTAL BARS MUST BE CONTINUOUS. DUE TO VARIABLES INVOLVED DURING INSTALLATION, IT SHALL BE THE CUSTOMER'S OR INSTALLER'S RESPONSIBILITY TO PROVIDE STRUCTURALLY ADEQUATE SUPPORTS FOR BASE AND ANCHOR CONNECTIONS. IT MAY ALSO BE NECESSARY FOR THE CUSTOMER OR INSTALLER TO SECURE THE SERVICE OF A LOCAL ENGINEER TO DETERMINE THAT INSTALLATION COMPLIES WITH LOCAL BUILDING CODES.
4. ADDITIONAL CORROSION PROTECTION MAY BE REQUIRED FOR STEEL GUY ANCHORS IN DIRECT CONTACT WITH SOIL.

CONCRETE ANCHOR BLOCK DATA FOR ANSI/TIA-222-G PRESUMPTIVE CLAY SOIL									
BLOCK	ANCHOR DIMENSIONS (IN.)			HORIZONTAL BARS QTY./SIZE	STIRRUPS SIZE & SPACING	CONCRETE VOL. (CU. YDS.)	UPLIFT CAPACITY(LBS)	LATERAL CAPACITY(LBS)	
	A	B	C						
AB1	3'-0"	1'-0"	3'-0"	(4) #5 BARS TOTAL (0) ADDITIONAL BAR EACH SIDE	#3 @ 12" O.C.	0.44 PER BLOCK 1.3 TOTAL FOR 3	4,800	2,150	
AB2	4'-0"	1'-6"	4'-0"	(10) #6 BARS TOTAL (0) ADDITIONAL BAR EACH SIDE	#3 @ 12" O.C.	1.33 PER BLOCK 4.0 TOTAL FOR 3	12,600	6,480	
AB3	6'-0"	1'-6"	3'-0"	(8) #6 BARS TOTAL (0) ADDITIONAL BAR EACH SIDE	#3 @ 12" O.C.	1.0 PER BLOCK 3.0 TOTAL FOR 3	18,700	10,500	
AB4	6'-0"	1'-6"	4'-0"	(10) #6 BARS TOTAL (0) ADDITIONAL BAR EACH SIDE	#4 @ 12" O.C.	2.0 PER BLOCK 6.0 TOTAL FOR 3	32,500	15,800	
AB5	8'-0"	2'-0"	3'-0"	(10) #7 BARS TOTAL (1) ADDITIONAL BAR EACH SIDE	#4 @ 12" O.C.	2.22 PER BLOCK 6.7 TOTAL FOR 3	43,000	21,000	
AB6	8'-0"	2'-0"	4'-0"	(12) #7 BARS TOTAL (1) ADDITIONAL BAR EACH SIDE	#4 @ 12" O.C.	2.96 PER BLOCK 8.9 TOTAL FOR 3	52,000	26,500	

(SEE TOWER ASSEMBLY DRAWING FOR ANCHOR ROD SLOPE 'E')

STANDARD FOUNDATION NOTES
ANSI/TIA-222-G

1. STANDARD FOUNDATION DESIGNS ARE IN ACCORDANCE WITH ANSI/TIA-222-G, "STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES", SECTION 9 AND ANNEX F FOR THE FOLLOWING PRESUMPTIVE CLAY SOIL PARAMETERS:

N (blows/ft) [blows/m]	Φ (deg)	Y (lb/ft ³) [kN/m ³]	C (psf) [kPa]	Ultimate Bearing (psf) [kPa]		Ultimate Skin Friction (psf) [kPa]	k (pcf) [kN/m ³]	e _s
				Shallow Frds.	Deep Frds.			
8 [26]	0	110 [17]	1000 [48]	5000 [240]	9000 [431]	500 [24]	150 [41,000]	0.01

2. THE PURCHASER MUST VERIFY THAT ACTUAL SITE SOIL PARAMETERS MEET OR EXCEED ANSI/TIA-222-G PRESUMPTIVE CLAY SOIL DESIGN PARAMETERS AND THAT THE PENETRATION AND/OR ZONE OF SEASONAL MOISTURE VARIATION AT THE SITE, FOUNDATION DESIGN MODIFICATIONS MAY BE REQUIRED IN THE EVENT PRESUMPTIVE CLAY SOIL PARAMETERS ARE NOT APPLICABLE FOR THE ACTUAL SUBSURFACE CONDITIONS ENCOUNTERED.
3. A SITE-SPECIFIC INVESTIGATION IS REQUIRED FOR CLASS III STRUCTURES IN ACCORDANCE WITH ANSI/TIA-222-G.
4. FOUNDATION DESIGNS ASSUME FIELD INSPECTIONS WILL BE PERFORMED BY THE PURCHASER'S REPRESENTATIVE TO VERIFY THAT CONSTRUCTION MATERIALS, INSTALLATION METHODS AND ASSUMED DESIGN PARAMETERS ARE ACCEPTABLE BASED ON THE CONDITIONS EXISTING AT THE SITE.
5. WORK SHALL BE IN ACCORDANCE WITH LOCAL CODES, SAFETY REGULATIONS AND UNLESS OTHERWISE NOTED, THE LATEST REVISION OF ACT 318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". PROCEDURES FOR THE PROTECTION OF EXCAVATIONS, EXISTING CONSTRUCTION AND UTILITIES SHALL BE ESTABLISHED PRIOR TO FOUNDATION INSTALLATION.
6. CONCRETE MATERIALS SHALL CONFORM TO THE APPROPRIATE STATE REQUIREMENTS FOR EXPOSED STRUCTURAL CONCRETE.
7. PROPORTIONS OF CONCRETE MATERIALS SHALL BE SUITABLE FOR THE INSTALLATION METHOD UTILIZED AND SHALL RESULT IN DURABLE CONCRETE FOR RESISTANCE TO LOCAL ANTICIPATED AGGRESSIVE ACTIONS. THE DURABILITY REQUIREMENT OF ACT 318 CHAPTER 4 SHALL BE SATISFIED BASED ON THE CONDITIONS EXPECTED AT THE SITE. AS A MINIMUM, CONCRETE SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI (27.6 MPa) IN 28 DAYS.
8. MAXIMUM SIZE OF AGGREGATE SHALL NOT EXCEED SIZE SUITABLE FOR INSTALLATION METHOD UTILIZED OR 1/3 CLEAR DISTANCE BEHIND OR BETWEEN REINFORCING. MAXIMUM SIZE MAY BE INCREASED TO 2/3 CLEAR DISTANCE PROVIDED WORKABILITY AND METHODS OF CONSOLIDATION SUCH AS VIBRATING WILL PREVENT HONEYCOMBS OR VOIDS.
9. REINFORCEMENT SHALL BE DEFORMED AND CONFORM TO THE REQUIREMENTS OF ASTM A615 UNLESS OTHERWISE INDICATED.
10. REINFORCING CAGES SHALL BE BRACED TO RETAIN PROPER DIMENSIONS DURING HANDLING, THROUGHOUT PLACEMENT OF CONCRETE AND DURING EXTRACTION OF TEMPORARY CASING.
11. WELDING IS PROHIBITED ON REINFORCING STEEL AND EMBEDMENTS.

12. MINIMUM CONCRETE COVER FOR REINFORCEMENT SHALL BE 3 INCHES (76 mm) UNLESS OTHERWISE NOTED. APPROVED SPACERS SHALL BE USED TO INSURE A 3 INCH (76 mm) MINIMUM COVER ON REINFORCEMENT. CONCRETE COVER FROM TOP OF FOUNDATION TO ENDS OF VERTICAL REINFORCEMENT SHALL NOT EXCEED 3 INCHES (76 mm) NOR BE LESS THAN 2 INCHES (51 mm).
13. SPACERS SHALL BE ATTACHED INTERMITTENTLY THROUGHOUT THE ENTIRE LENGTH OF VERTICAL REINFORCING CAGES TO INSURE CONCENTRIC PLACEMENT OF CAGES IN EXCAVATIONS.
14. FOUNDATION DESIGNS ASSUME STRUCTURAL BACKFILL TO BE COMPACTED IN 8 INCH (200 mm) MAXIMUM LAYERS TO 95% OF MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT IN ACCORDANCE WITH ASTM D698. ADDITIONALLY, STRUCTURAL BACKFILL MUST HAVE A MINIMUM COMPACTED UNIT WEIGHT OF 100 POUNDS PER CUBIC FOOT (16 kN/m³).
15. FOUNDATION DESIGNS ASSUME LEVEL GRADE AT THE SITE.
16. FOUNDATION INSTALLATION SHALL BE SUPERVISED BY PERSONNEL KNOWLEDGEABLE AND EXPERIENCED WITH THE PROPOSED FOUNDATION TYPE. CONSTRUCTION SHALL BE IN ACCORDANCE WITH GENERALLY ACCEPTED INSTALLATION PRACTICES.
17. FOR FOUNDATION AND ANCHOR TOLERANCES SEE DRAWING A810214.
18. LOOSE MATERIAL SHALL BE REMOVED FROM BOTTOM OF EXCAVATION PRIOR TO CONCRETE PLACEMENT. SIDES OF EXCAVATION SHALL BE ROUGH AND FREE OF LOOSE CUTTINGS.
19. CONCRETE SHALL BE PLACED IN A MANNER THAT WILL PREVENT SEGREGATION OF CONCRETE MATERIALS, INFILTRATION OF WATER OR SOIL AND OTHER OCCURRENCES WHICH MAY DECREASE THE STRENGTH OR DURABILITY OF THE FOUNDATION.
20. FREE FALL CONCRETE MAY BE USED PROVIDED FALL IS VERTICAL DOWN WITHOUT HITTING SIDES OF EXCAVATION, FORMWORK, REINFORCING BARS, FORM TIES, CAGE BRACING OR OTHER OBSTRUCTIONS. UNDER NO CIRCUMSTANCES SHALL CONCRETE FALL THROUGH WATER.
21. CONCRETE SHALL BE PLACED AGAINST UNDISTURBED SOIL EXCEPT FOR PIERS OR PIER AND PAD FOUNDATIONS. FORMS FOR PIERS SHALL BE REMOVED PRIOR TO PLACING STRUCTURAL BACKFILL.
22. CONSTRUCTION JOINTS, IF REQUIRED IN PIER MUST BE AT LEAST 12 INCHES (305 mm) BELOW BOTTOM OF EMBEDMENTS AND MUST BE INTENTIONALLY ROUGHENED TO A FULL AMPLITUDE OF 1/4 INCH (6 mm). FOUNDATION DESIGN ASSUMES NO OTHER CONSTRUCTION JOINTS.
23. CASING, IF USED, SHALL NOT BE LEFT IN PLACE. EQUIPMENT, PROCEDURES, AND PROPORTIONS OF CONCRETE MATERIALS SHALL INSURE CONCRETE WILL NOT BE ADVERSELY DISTURBED UPON CASING REMOVAL. DRILLING FLUID, IF USED, SHALL BE FULLY DISPLACED BY CONCRETE AND SHALL NOT BE DETRIMENTAL TO CONCRETE OR SURROUNDING SOIL. CONTAMINATED CONCRETE SHALL BE REMOVED FROM TOP OF FOUNDATION AND REPLACED WITH FRESH CONCRETE.
24. TOP OF FOUNDATION SHALL BE SLOPED TO DRAIN WITH A FLOATED FINISHED. EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4" X 3/4" (19 mm X 19 mm) MINIMUM.
25. FOR ANCHOR BLOCK TYPE FOUNDATIONS, FOR GUYED TOWERS, ADDITIONAL CORROSION PROTECTION MAY BE REQUIRED FOR STEEL GUY ANCHORS IN DIRECT CONTACT WITH SOIL. DESIGN ASSUMES PERIODIC INSPECTIONS WILL BE PERFORMED OVER THE LIFE OF THE STRUCTURE TO DETERMINE IF ADDITIONAL ANCHOR CORROSION PROTECTION MEASURES MUST BE IMPLEMENTED BASED ON OBSERVED SITE-SPECIFIC CONDITIONS.

FILE NO.

REVISIONS			
REV.	DESCRIPTION	DWN	CHK
1	DATE: 08/2012	JEC	JDN



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ANSI/TIA-222-G
STANDARD FOUNDATION NOTES

DWN:	PAID	CHKD:	HA	DATE:	Nov/20/2009
ENGR:			HA	SHEET #:	1 OF 1
PRO. ENGR:				PRO. MGR:	

DRAWING NO:	B090548	REV:	1
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FILE NO.

Standard-80

REVISIONS			
REV	DESCRIPTION	DWN	CHK APP
2	REDRAWN IN AUTOCAD	M.F.	JDM H.A.
3	DATE: 08/22/2008 REVISION: 08/22/2008 REASON: ANCHOR INSERTION NOTE TO STANDARD	JMS	H.A.
	DATE: 02/15/2010		

DWG REFERENCE

ROHN

PRODUCTS

6718 WEST PLANK ROAD

PEORIA, IL 61604

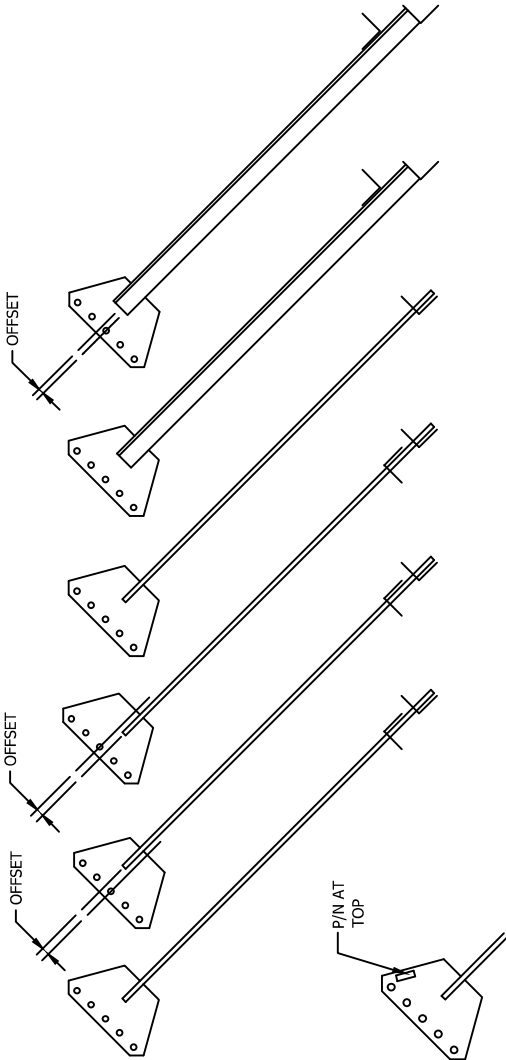
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ANCHOR INSTALLATION DETAIL
ANCHOR INSTALLATION DETAIL

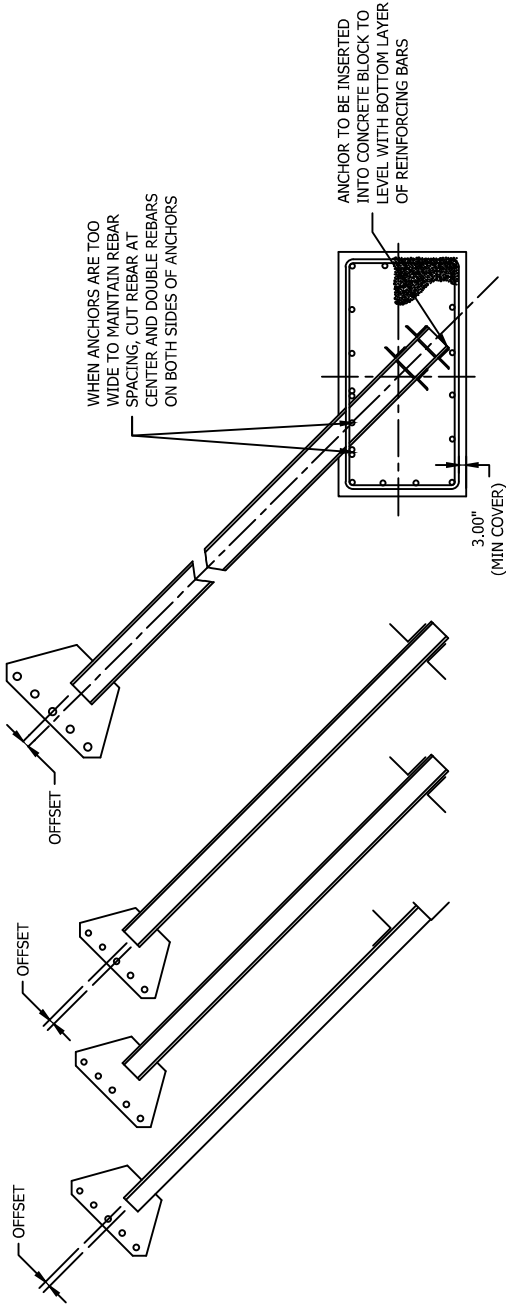
DWN:	B.N.	CHKD:	W.D.U.	DATE:	08/22/1995
ENGR:					

DRAWING NO:	A951409	REV:	3
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NOTE:
INSTALL ANCHORS IN BLOCKS WITH EMBEDMENT
ANGLES ORIENTED AS SHOWN AND WITH P/N AT
TOP AS SHOWN.

TYPICAL DETAIL



FILE NO.

Standard-80

REVISIONS

REV	DESCRIPTION	DWG	CHK	APP
1	REBRAIN INTO AUTOCAD & AUGED W/IC #2		JDA	JDA
4	DATE: Nov/22/2006			

DWG REFERENCE

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GUY ANCHOR

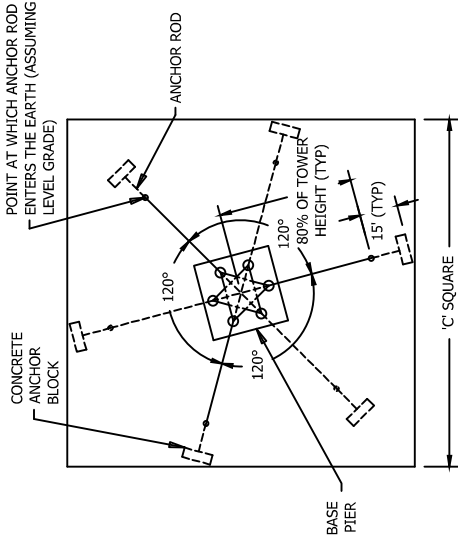
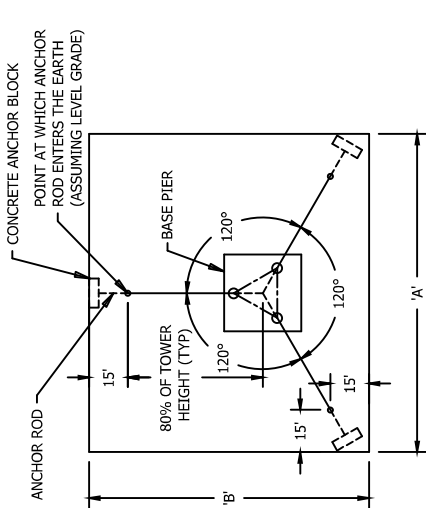
LAND REQUIREMENT DETAIL

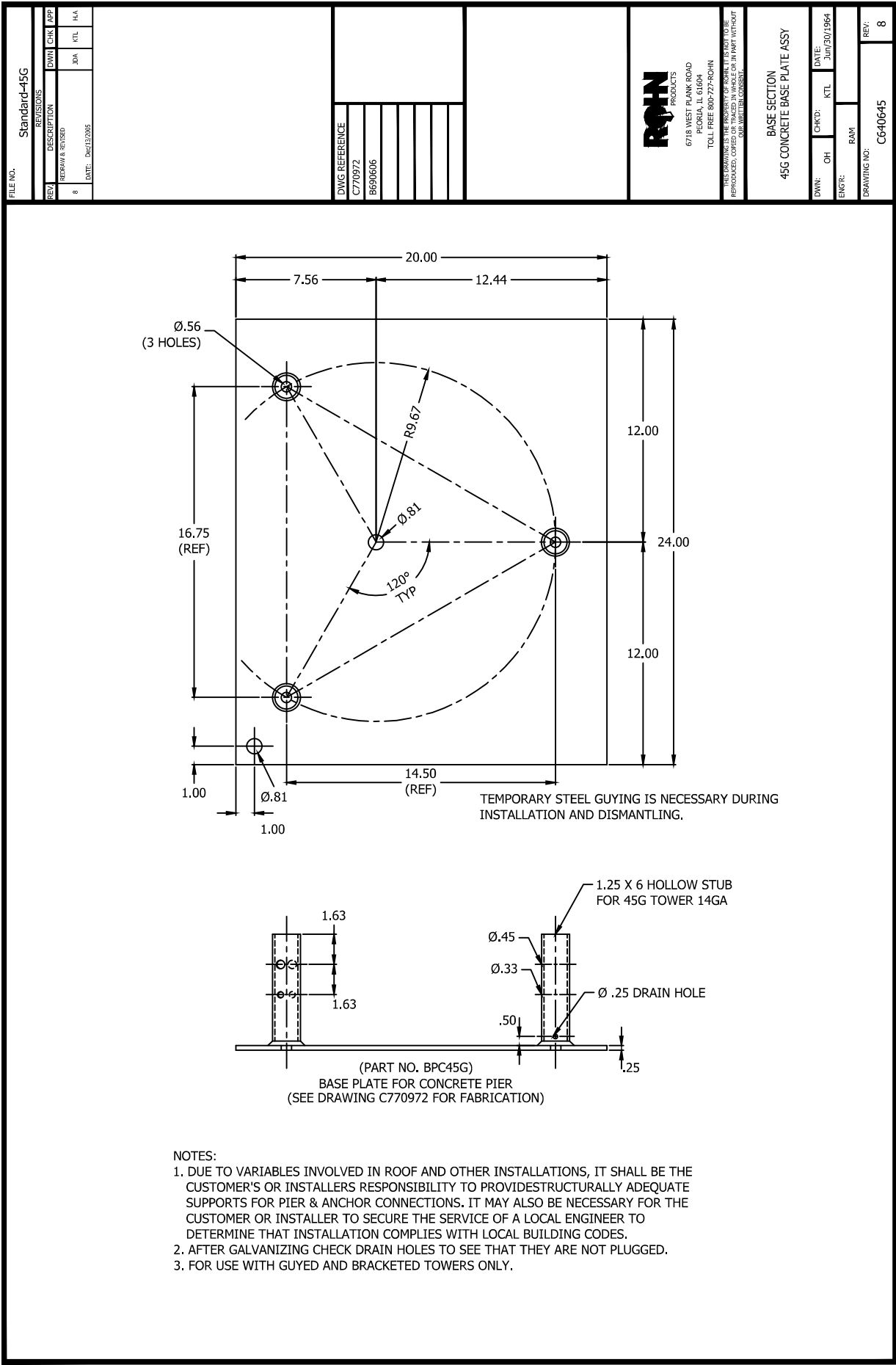
DWGN: MSK
ENGR: CW
DRAWING NO: C640531
REV: 4

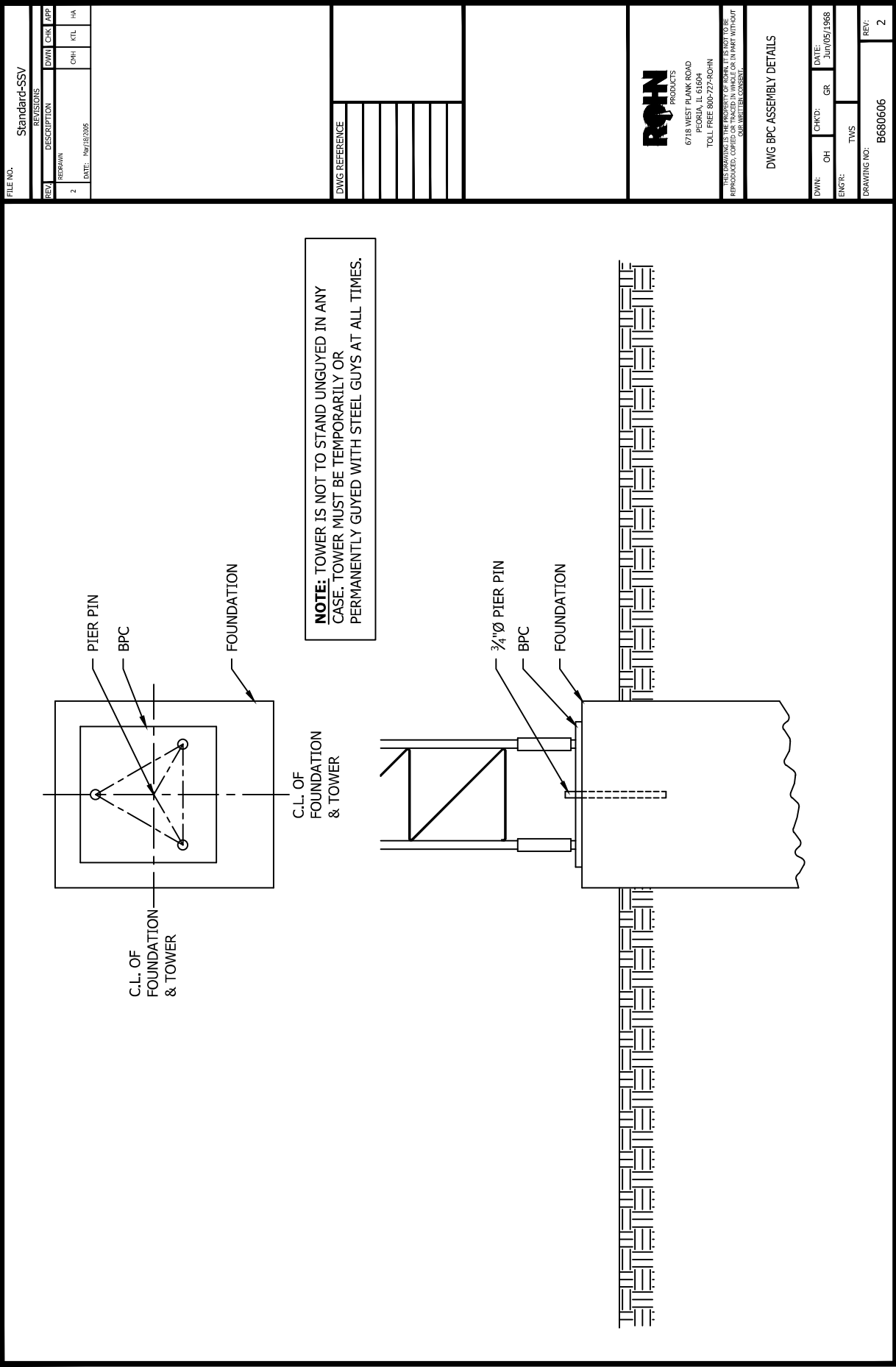
TOWER HEIGHT	LAYOUT A			LAYOUT B		
	ACRES	A	B	ACRES	C	
20'	0.08	60'	55'	0.10	65'	
30'	0.12	75'	70'	0.15	80'	
40'	0.17	90'	80'	0.21	95'	
50'	0.21	100'	90'	0.28	110'	
60'	0.28	115'	105'	0.39	130'	
70'	0.35	130'	115'	0.48	145'	
80'	0.43	145'	130'	0.59	160'	
90'	0.50	155'	140'	0.70	175'	
100'	0.59	170'	150'	0.83	190'	
110'	0.70	185'	165'	1.01	210'	
120'	0.80	200'	175'	1.16	225'	
130'	0.94	215'	190'	1.32	240'	
140'	1.04	225'	200'	1.49	255'	
150'	1.16	240'	210'	1.67	270'	
160'	1.32	255'	225'	1.93	290'	
170'	1.46	270'	235'	2.14	305'	
180'	1.64	285'	250'	2.35	320'	
190'	1.76	295'	260'	2.58	335'	
200'	1.92	310'	270'	2.81	350'	
210'	2.13	325'	285'	3.14	370'	
220'	2.31	340'	295'	3.40	385'	
230'	2.50	350'	310'	3.67	400'	
240'	2.68	365'	330'	3.95	415'	
250'	2.88	380'	330'	4.24	430'	
260'	3.13	395'	345'	4.65	450'	
270'	3.34	410'	355'	4.96	465'	
280'	3.57	420'	370'	5.29	480'	
290'	3.80	435'	380'	5.63	495'	
300'	4.03	450'	390'	5.97	510'	
310'	4.33	465'	405'	6.45	530'	
320'	4.53	475'	415'	6.82	545'	
330'	4.84	490'	430'	7.20	560'	
340'	5.10	505'	440'	7.59	575'	
350'	5.37	520'	450'	8.00	590'	
360'	5.71	535'	465'	8.54	610'	
370'	5.94	545'	475'	8.97	625'	
380'	6.30	560'	490'	9.40	640'	
390'	6.60	575'	500'	9.85	655'	
400'	6.91	590'	510'	10.31	670'	
410'	7.23	600'	525'	10.93	690'	
420'	7.55	615'	535'	11.41	705'	
430'	7.96	630'	550'	11.90	720'	
440'	8.29	645'	560'	12.40	735'	
450'	8.64	660'	570'	12.91	750'	
460'	9.00	670'	585'	13.61	770'	
470'	9.36	685'	595'	14.15	785'	
480'	9.80	700'	610'	14.69	800'	
490'	10.18	715'	620'	15.25	815'	
500'	10.49	725'	630'	15.81	830'	

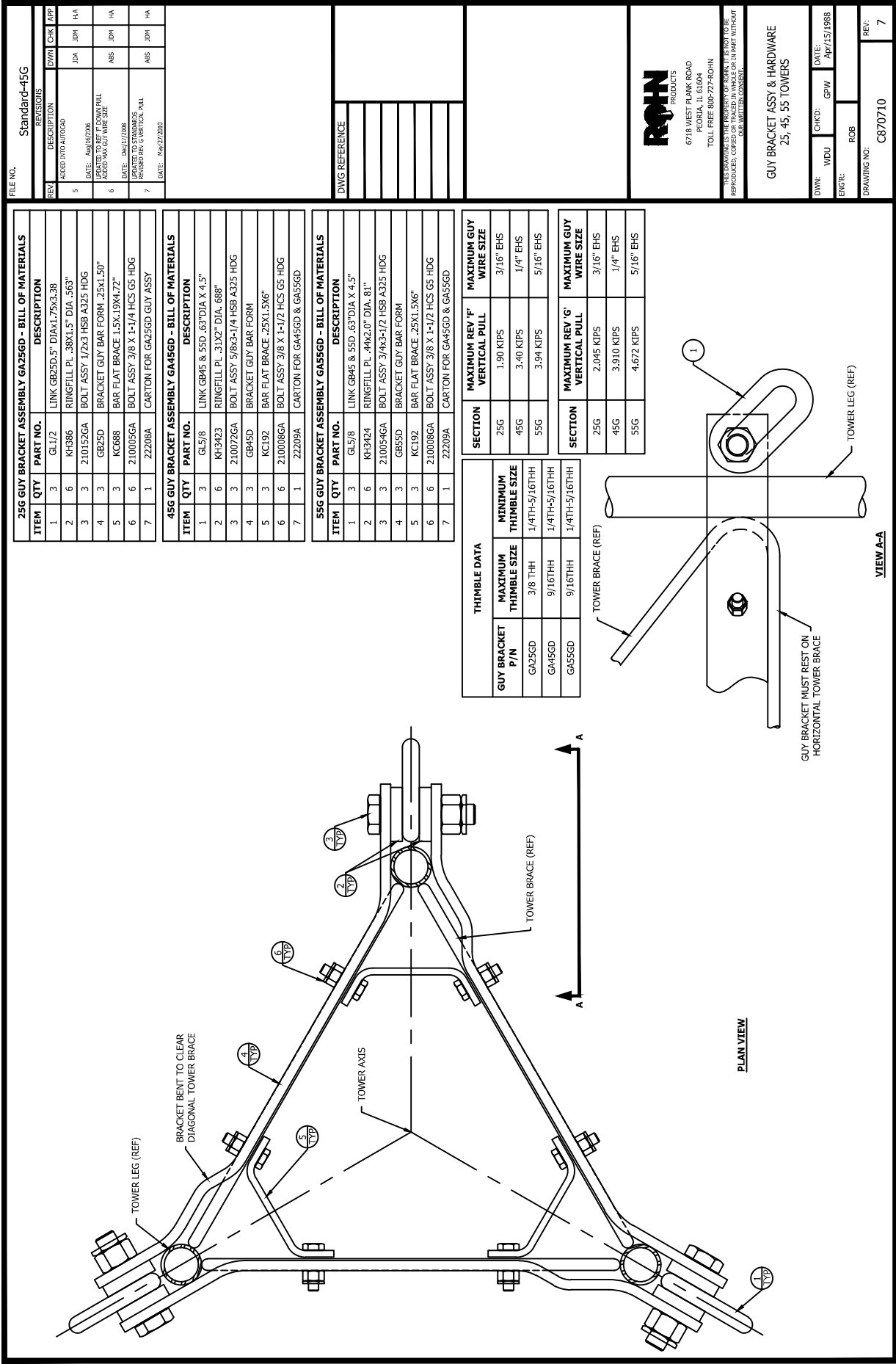
GENERAL NOTES

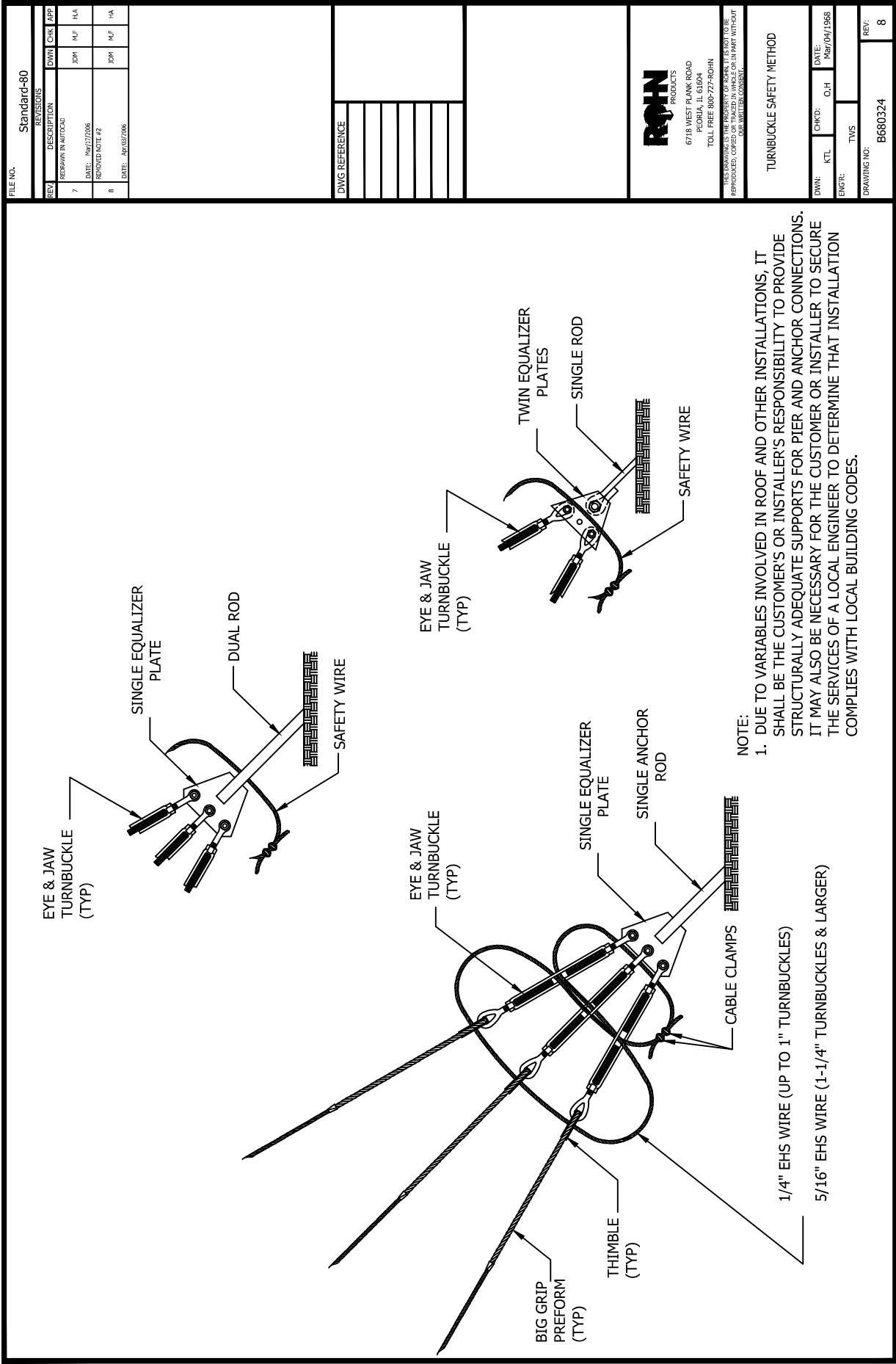
1. DUE TO VARIABLES INVOLVED IN ROOF AND OTHER INSTALLATIONS, IT SHALL BE THE RESPONSIBILITY OF THE CUSTOMER OR INSTALLER TO PROVIDE STRUCTURALLY ADEQUATE SUPPORTS FOR PIER AND ANCHOR CONNECTIONS. IT MAY ALSO BE NECESSARY FOR THE CUSTOMER OR INSTALLER TO SECURE THE SERVICE OF A LOCAL ENGINEER TO DETERMINE THAT THE INSTALLATION COMPLIES WITH LOCAL BUILDING CODES.
2. FOR RESTRICTED SITES, CUSTOM DESIGNS WITH STRONGER MASTS AND LARGER GUYS MAY BE PROVIDED BY REDUCING THE GUY RADIUS FROM 80% TO 40% OF THE TOWER HEIGHT.











FILE NO.

Standard-90

REVISIONS

REV	DESCRIPTION	DWM	CHK	APP
3	REBRWAW INTO AUTOCAD	MEF	JDH	HA
4	DATE: 05/04/2006 REVISED NOTATION		JDH	HA
	DATE: 09/13/2007			

DWG REFERENCE

ROHN

PRODUCTS

6718 WEST PLANK ROAD
PEORIA, IL 61604
TOLL FREE 800-727-ROHN

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OUR WRITTEN CONSENT.

GUY

WIRE HARDWARE KIT

DWN: WHW

CHKD: WJM

DATE: 5sep/30/1987

ENGR: RDM

DRAWING NO: A871382

REV: 4

WIRE SIZE	ANCHOR ROD	TURNBUCKLE	THIMBLE
3/16 EHS	GAR30	5/8TBE&J	5/16THH
	GAC303,305	3/8TBE&E	5/16THH
	GAC3455	1/2TBE&J	5/16THH
	GAC5655	5/8TBE&J	5/16THH
1/4 EHS	GAR30	5/8TBE&J	3/8THH
	GAC303,305	1/2TBE&E	3/8THH
	GAC3455	1/2TBE&J	3/8THH
	GAC5655	5/8TBE&J	3/8THH
5/16 EHS	GAR30	5/8TBE&J	7/16THH
	GAC303,305	5/8TBE&J	7/16THH
	GAC3455	5/8TBE&J	7/16THH
	GAC5655	5/8TBE&J	7/16THH
3/8 EHS	GAR30	5/8TBE&J	1/2THH
	GAC3455	5/8TBE&J	1/2THH
	GAC5655	5/8TBE&J	1/2THH

FILE NO.

Standard-80

REVISIONS

REV	DESCRIPTION	DWN	CHK	APP
3	REDRAWN IN AUTOCAD	JDM	ME	H.A
DATE: May 17/2006				

DWG REFERENCE

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SPLICE CONNECTION

FOR BIG GRIP & END SLEEVE

DWN: H.A

CHKD: RAM

DATE: Jun/09/1970

ENGR: TWS

DRAWING NO. B700607

REV: 3

TO ACHIEVE MAXIMUM COVERAGE WITH THE END SLEEVE, THE APPLICATION SHOULD BE CONDUCTED IN THE FOLLOWING MANNER



1

PLACE THE SLOT SIDE OF THE
END SLEEVE OVER THE LONG
LEG OF THE DEAD END



2

DRIVE THE SLEEVE DOWNWARD UNTIL
THE RODS OF THE SHORT LEG ARE
COMPLETELY COVERED



3

THE RODS OF THE LONG LEG
SHOULD BE EVEN WITH, OR
MAY EXTEND ABOVE, THE TOP
EDGE OF THE SLEEVE

BE SURE TO SELECT THE PROPER SIZE END SLEEVE

FILE NO.

Standard-80

REVISIONS

REV	DESCRIPTION	DWN	CHK	APP
2	REDRAWN TO AUTOCAD	JDA	JDM	HAA
DATE: JUL/19/2006				

DWG REFERENCE

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PEORIA, IL 61604

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ANCHOR ATTACHMENT DETAIL

DWN: AJG

CHKD: WDU

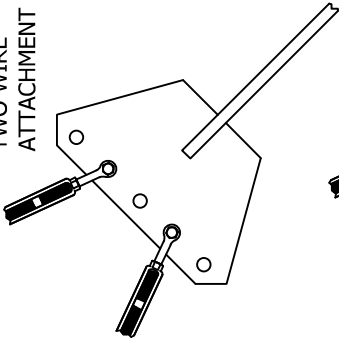
DATE: JUN/23/1982

ENGR: RAM

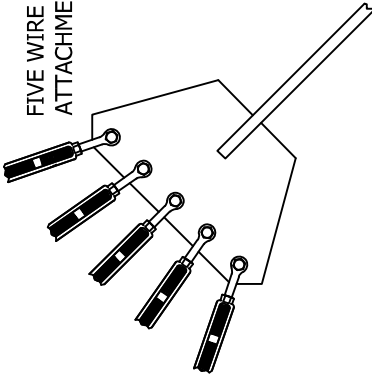
DRAWING NO. B820511

REV. 2

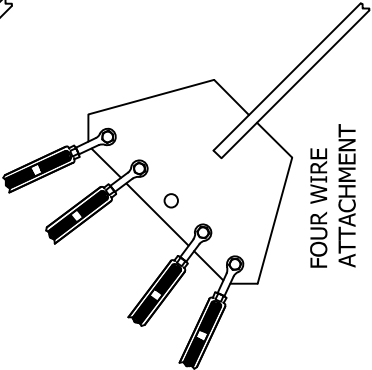
TWO WIRE ATTACHMENT



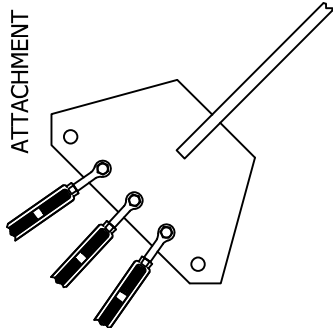
FIVE WIRE ATTACHMENT



FOUR WIRE ATTACHMENT

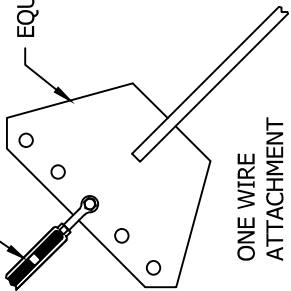


THREE WIRE ATTACHMENT



TURNBUCKLE (TYP)

EQUALIZER (TYP)



ONE WIRE ATTACHMENT

NOTE: SEE TOWER ASSEMBLY DRAWING FOR SIZE AND QTY OF TURNBUCKLES REQUIRED.

